

**Department of Mechanical, Mechatronics & Manufacturing Engineering,  
University City Campus (KSK), UET Lahore.**

<b>Subject:</b>	<b>Mechanics of Materials-I</b>	<b>End Term Exam KEY</b>
<b>Time Allowed: 2 hours</b>	<b>Max. Marks: 40</b>	<b>Session 2008</b>
<b>Name: _____</b>	<b>Reg.# 2008-EP- _____</b>	<b>Date: 09-01-2010</b>

**Note: Attempt all questions. E = 200 GPa For Steel.**

**1 (a)** Determine moment of Inertia ( $I_y$ ) along Y-Axis for the area shown in **Fig (1a)**.

The X-section can be considered as three composite rectangular areas A, B & D as shown

**Rectangle A & D:**

$$I_y = \frac{1}{12}(300 \text{ mm})(100 \text{ mm})^3 + (100 \text{ mm})(300 \text{ mm})(250 \text{ mm})^2$$

$$= 1.90 * 10^9 \text{ mm}^4$$

**Rectangle B :**

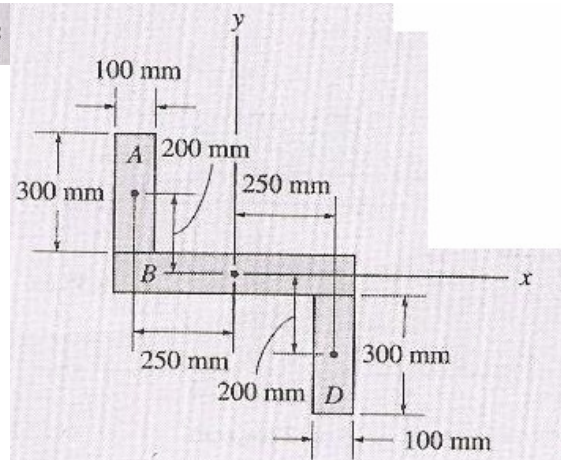
$$I_y = \frac{1}{12}(100 \text{ mm})(600 \text{ mm})^3$$

$$= 1.80 * 10^9 \text{ mm}^4$$

so,

$$I_y = 1.90(10^9) + 1.80(10^9) + 1.90(10^9)$$

$$= 5.60(10^9) \text{ mm}^4 \quad \text{Ans.}$$



**1 (b)** If the magnitude of the couples required to bend the steel strip is  $4.32 * 10^{-3}$  N. m into a circle of 20 mm diameter without resulting permanent deformation, then find the width "w" of the strip as shown in **Fig. (1b)**.

**DATA:**

$$M = 4.32 * 10^{-3} \text{ N.m}$$

$$d = 20 \text{ mm} = 20 * 10^{-3} \text{ m OR } r = 10 * 10^{-3} \text{ m}$$

$$w = ?$$

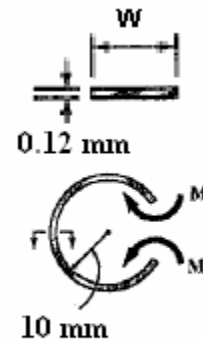
$$h = 0.12 \text{ mm OR } 0.12 * 10^{-3} \text{ m}$$

From Appendix: For steel  $E = 200 \text{ GPa}$ .

**Solution:**

$$\text{Flexure formula: } M / I = E / r \text{ ----- (1)}$$

$$\text{Since, } I = wh^3 / 12$$



**Fig. (1b)**

$$(1) \rightarrow 12M / wh^3 = E / r$$

$$\rightarrow 12M r / E h^3 = w$$

$$\rightarrow w = 12 * 4.32 * 10^{-3} \text{ N.m} * 10 * 10^{-3} \text{ m} / 200 \text{ GPa} * (0.12 * 10^{-3} \text{ m})^3$$

$$\rightarrow w = 0.15 \text{ m} \quad \text{Ans.}$$

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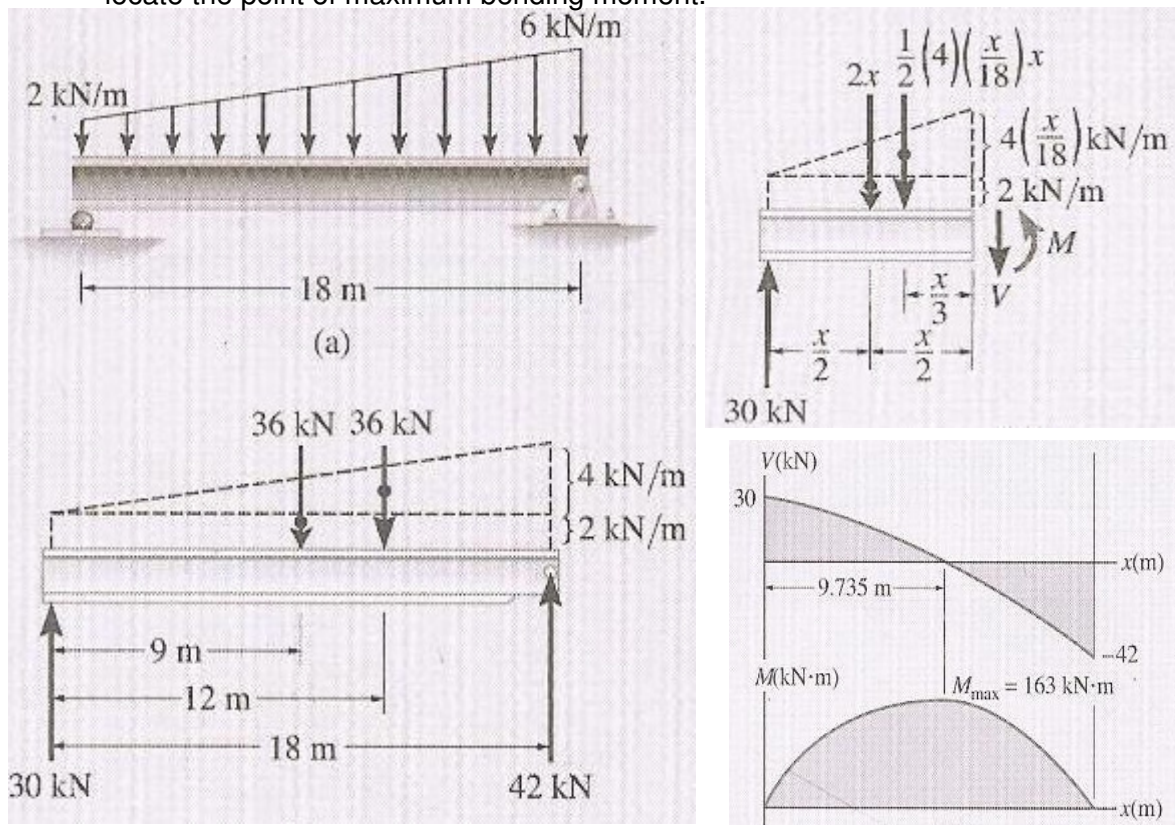
- 2 (a) from the table in appendix the column's  
 Area =  $A = 5890 \text{ mm}^2$ ,  $I_x = 45.5 \cdot 10^6 \text{ mm}^4$ ,  $I_y = 15.3 \cdot 10^6 \text{ mm}^4$   
 By inspection buckling will occur about Y – Axis.  
 So,  $P_{cr} = \pi^2 EI / (ZL)^2 = 1887.6 \text{ KN}$   
 Now,  $\sigma_{cr} = P_{cr} / A = 320.5 \text{ MPa}$ .  
 Since this stress exceeds the yield stress of steel ( 250 MPa), the load P  
 is determined from simple compression.

$$250 \text{ MPa} = P / 5890 \text{ mm}^2$$

$$\rightarrow P = 1472.5 \text{ KN. Ans.}$$

- 2 (b) Flexure Rigidity =  $E I = 80 \cdot 10^9 \text{ N-mm}^2 = 80 \cdot 10^9 \cdot (10^{-3})^2 \text{ N-m}^2$   
 $Y = 5 WL^4 / 384 E I = 5 \cdot 2 \cdot 4^4 / 384 \cdot 80 \cdot 10^9 (10^{-3})^2 = 83.3 \cdot 10^{-3} \text{ m}$   
**Derivation: AS in Class.**

- Q#3** Draw shear force & bending moment diagrams for the beam shown in **Fig. (3)** & locate the point of maximum bending moment.



- Q#4** (a) Derive the relation for crippling load using Euler's Theorem of Columns. Assuming that the column is pinned connected at its ends.  
 (b) Define: Perpendicular axis theorem, Bifurcation point, Section modulus & Deflection of beam.

**AS IN CLASS LECTURE.**